



1 **Fractional governing equations of transient groundwater flow in unconfined**  
2 **aquifers with multi-fractional dimensions in fractional time**

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13 **Abstract:** In this study, a dimensionally-consistent governing equation of transient unconfined  
14 groundwater flow in fractional time and multi-fractional space is developed. First, a fractional  
15 continuity equation for transient unconfined groundwater flow is developed in fractional time and  
16 space. For the equation of groundwater motion within a multi-fractional multi-dimensional  
17 unconfined aquifer, a previously-developed dimensionally consistent equation for water flux in  
18 unsaturated/saturated porous media is combined with the Dupuit approximation to obtain an  
19 equation for groundwater motion in multi-fractional space in unconfined aquifers. Combining the  
20 fractional continuity and groundwater motion equations, the fractional governing equation of  
21 transient unconfined aquifer flow is then obtained. Finally, a numerical application to an  
22 unconfined aquifer groundwater flow problem is presented to show the skills of the proposed  
23 fractional governing equation.

24 **1. Introduction**

25 One way to obtain non-Fickian behavior in solute transport is by treating the underlying flow  
26 field to have long-range dependence in time (Kim et al., 2015; Kavvas et al., 2015). As shown by



27 Ercan and Kavvas (2014, and 2017), such dependence in time can be modeled by a time-fractional  
28 governing equation of the specified flow field. Flow velocity correlation and distribution in  
29 fractured media, which can be modeled by Continuous Time Random Walk models (Metzler and  
30 Klafter, 2000), may also result in non-Fickian transport (Kang et al., 2015). Long-range  
31 dependence in time reported in groundwater level fluctuations (e.g., Li and Zhang, 2007; Yu et al.,  
32 2016; Tu et al., 2017; and the references therein) and anisotropy in aquifer medium necessitates  
33 time- and space-fractional operators in the governing equations of groundwater flow (Kavvas et  
34 al., 2017a).

35 Reporting that conventional geometries cannot characterize groundwater flow in many  
36 fractured rock aquifers (Black et al., 1986), and the observed drawdown tends to be underestimated  
37 in early times and overestimated at later times by the conventional radial groundwater flow model  
38 (Van Tonder et al., 2001), Clout and Botha (2006) developed a fractional governing equation for  
39 radial groundwater flow in integer time and fractional space in a uniform homogeneous aquifer.  
40 They used the Riemann-Liouville fractional derivative form in the model formulation. Atangana  
41 and Bildik (2013), Atangana (2014), and Atangana and Vermeulen (2014) then reformulated the  
42 fractional radial groundwater flow model of Clout and Botha (2006) by the Caputo differentiation  
43 framework, and reported better performance. Compared to the Riemann-Liouville derivative  
44 approach, the Caputo framework has a fundamental advantage of being able to accommodate  
45 physically-interpretable real-life initial and boundary conditions (Podlubny, 1998). Atangana and  
46 Baleanu (2014) presented a new radial groundwater flow model in fractional time based on a new  
47 fractional derivative definition, "conformable derivative" (Khalil et al., 2014). Most recently, Su  
48 (2017) proposed a time-space fractional Boussinesq equation and he claimed this fractional  
49 equation is a general groundwater flow equation and can be applied to groundwater flow in both  
50 confined and unconfined aquifers. However, all of the aforementioned studies only presented the  
51 formulated fractional governing groundwater flow equations and no detailed derivations of these  
52 governing equations from the fundamental conservation principles were provided.

53 Wheatcraft and Meerschaert (2008) derived the groundwater flow continuity equation in the  
54 fractional form by using the fractional Taylor series approximation. They further removed the  
55 linearity / piecewise linearity restriction for the flux and the infinitesimal control volume  
56 restriction. When developing the fractional continuity equation, the groundwater flow process was  
57 considered in fractional space but in integer time by Wheatcraft and Meerschaert (2008). They



58 further assumed the same fractional power in every direction of the fractional porous media space.  
59 Furthermore, only the mass conservation was considered in their derivation, but not the fractional  
60 water flux equation. Mehdejadani et al. (2013) expanded the approach of Wheatcraft and  
61 Meerschaert (2008) to the derivation of a governing equation of groundwater flow in an  
62 unconfined aquifer in fractional space but in integer time. In their derivation, they used the  
63 conventional Darcy formulation for the water flux with integer spatial derivative while utilizing  
64 fractional spatial derivatives in their continuity equation.

65 Olsen et al. (2016) pointed out that the derivations in Wheatcraft and Meerschaert (2008) and  
66 Mehdejadani et al. (2013) utilized the fractional Taylor series, as formulated by Odibat and  
67 Shawagfeh (2007), which utilized local Caputo derivatives. In order to expand the local Caputo  
68 derivatives in the above-mentioned studies, Olsen et al. (2016) utilized the fractional mean value  
69 theorem from Diethelm (2012) to develop a continuity equation of groundwater flow with left and  
70 right fractional nonlocal Caputo derivatives in fractional space but in integer time. Olsen et al.  
71 (2016) did not address the water flux formulation in fractional space, and, hence, did not develop  
72 a complete governing equation of groundwater flow. They also did not address the multifractional  
73 spatial derivatives in order to address anisotropy within an aquifer. Around that time, Kavvas et  
74 al. (2017a) utilized the mean value formulation from Usero (2007), Odibat and Shawagfeh (2007)  
75 and Li et al. (2009) to derive a complete governing equation of transient groundwater flow in a  
76 confined, anisotropic aquifer with fractional time and multi-fractional space derivatives which  
77 addressed not only the continuity but also the water flux (motion) in fractional time-space and the  
78 effect of a sink/source term. By employing the above-mentioned fractional mean value  
79 formulations, Kavvas et al. (2017a) developed the governing equation of confined groundwater  
80 flow in fractional time-space in non-local form.

81 Unconfined groundwater flow is the fundamental component of the watershed runoff  
82 baseflow since it is the fundamental contributor to the network streamflow within a watershed  
83 during dry periods. As such, the behavior of unconfined groundwater flow is key to the physically-  
84 based understanding of the long memory in watershed runoff. Meanwhile, as will be seen in the  
85 following derivation of its governing equation, unconfined aquifer groundwater flow is uniquely  
86 different from the confined aquifer groundwater flow. The fundamental differences between the  
87 two aquifer flows is that while the flow in a confined aquifer is linear and compressible, the flow  
88 in an unconfined aquifer is nonlinear and incompressible due to the unconfined aquifer being



89 phreatic, its top surface boundary being open to the atmosphere. Accordingly, hydrologists have  
90 developed unique governing equations of unconfined aquifer groundwater flow (Bear, 1979;  
91 Freeze and Cherry, 1979). Starting with the next section, first the continuity equation of transient  
92 unconfined groundwater flow within an anisotropic heterogeneous aquifer under a time-space  
93 varying sink/source will be developed in fractional time and fractional space. Then, this fractional  
94 continuity equation will be combined with a fractional groundwater motion equation to obtain a  
95 transient groundwater flow equation in fractional time-multifractional space within an anisotropic,  
96 heterogeneous unconfined aquifer.

97 Analogous to the traditional governing groundwater flow equations, as outlined by Freeze  
98 and Cherry (1979) and Bear (1979), the fractional unconfined groundwater flow equations must  
99 have specific features (Kavvas et al., 2017a):

100 i. In order for the governing equation to be prognostic, the form of the equation must be known  
101 completely from the outset.

102 ii. The fractional governing equations must be dimensionally consistent and be purely  
103 differential equations, containing only differential operators without difference operators.

104 iii. As the fractional derivative powers go to integer values, the fractional unconfined  
105 groundwater flow equations must converge to the corresponding conventional integer-order  
106 governing equations.

107 Within this framework, the governing equations of unconfined groundwater flow in fractional  
108 time and fractional space will be developed in the following.

## 109 **2. Derivation of the Continuity Equation for Transient Unconfined Groundwater Flow in a** 110 **Heterogeneous Anisotropic Multi-Fractional Medium in Fractional Time**

111 To  $\beta$ -order the Caputo fractional derivative  $D_a^{k\beta} f(x)$  of a function  $f(x)$  may be defined as  
112 ( Odibat and Shawagfeh, 2007; Podlubny, 1998; Usero, 2007, Li et al., 2009),  
113

$$114 \quad D_a^\beta f(x) = \frac{1}{\Gamma(1-\beta)} \int_a^x \frac{f(\xi)}{(x-\xi)^\beta} d\xi \quad 0 < \beta < 1, \quad x \geq a \quad . \quad (1)$$

115

116 It was shown in Kavvas et al. (2017b) that one can obtain a  $\beta_{x_i}$ -order approximation (i=1,2)  
117 to a function  $f(x_i)$  around  $x_i - \Delta x_i$  as



118

$$119 \quad f(x_i) = f(x_i - \Delta x_i) + \frac{(\Delta x_i)^{\beta_{x_i}}}{\Gamma(\beta_{x_i} + 1)} D_{x_i - \Delta x_i}^{\beta_{x_i}} f(x_i) \quad ; i=1,2. \quad (2)$$

120 In Equation (2), an analytical relationship between  $\Delta x_i$  and  $(\Delta x_i)^{\beta_{x_i}}$  ( $i=1,2$ ) that will be universally  
 121 applicable throughout the modelling domain is possible when the lower limit in the above Caputo  
 122 derivative in equation (2) is taken as zero (that is,  $\Delta x_i = x_i$ ) for  $f(x_i) = x_i$  (Kavvas et al. 2017b).

123 Under the Dupuit approximation of horizontal flow streamlines (very small water table  
 124 gradient) (Bear, 1979), the net mass flux through the control volume of an unconfined aquifer with  
 125 a flat bottom confining layer, as depicted in Figure 1, that also has a sink/source mass flux  
 126  $\rho q_v \Delta x_1 \Delta x_2$ , can be formulated as

127

$$128 \quad [\rho Q_{x_1}(x_1, x_2; t) - \rho Q_{x_1}(x_1 - \Delta x_1, x_2; t)] \Delta x_2 + [\rho Q_{x_2}(x_1, x_2; t) - \rho Q_{x_2}(x_1, x_2 -$$

$$129 \quad \Delta x_2; t)] \Delta x_1 - \rho q_v \Delta x_1 \Delta x_2 \quad (3)$$

130

131 where  $Q_{x_i}$  is the discharge across a vertical plane of unit width in  $i$ -th direction,  $i = 1,2$ ,  $\rho$  is the  
 132 fluid density, and  $q_v$  is the source/sink (recharge/leakage) per unit horizontal area. Then by  
 133 combining equation (2) with equation (3) with  $\Delta x_i = x_i$  ( $i=1,2$ ), and expressing the resulting  
 134 Caputo derivative  $D_0^{\beta_{x_i}} f(x_i)$  by  $\frac{\partial^{\beta_{x_i}} f(x_i)}{(\partial x_i)^{\beta_{x_i}}}$ , ( $i=1,2$ ) for convenience, yields the net mass flux  
 135 through the control volume in Figure 1 to the orders of  $(\Delta x_1)^{\beta_{x_1}}$  and  $(\Delta x_2)^{\beta_{x_2}}$ , as

$$\frac{1}{\Gamma(\beta_{x_1} + 1)} \left( \frac{\partial}{\partial x_1} \right)^{\beta_{x_1}} (\rho Q_{x_1}(x_1, x_2; t)) (\Delta x_1)^{\beta_{x_1}} \Delta x_2 +$$

$$\frac{1}{\Gamma(\beta_{x_2} + 1)} \left( \frac{\partial}{\partial x_2} \right)^{\beta_{x_2}} (\rho Q_{x_2}(x_1, x_2; t)) \Delta x_1 (\Delta x_2)^{\beta_{x_2}} - \rho q_v \Delta x_1 \Delta x_2 \quad (4)$$

136 where different powers for fractional space derivatives are utilized in different directions due to  
 137 the anisotropy in the flow medium.

138

139 Kavvas et al. (2017b) have shown that to  $\beta_{x_i}$ -order fractional increments in space in the  $i$ -th  
 140 direction,  $i=1,2$ ,



$$(\Delta x_i)^{\beta_{x_i}} = \frac{\Gamma(\beta_{x_i}+1)\Gamma(2-\beta_{x_i})}{x_i^{1-\beta_{x_i}}} \Delta x_i \quad , \quad i=1,2. \quad (5)$$

141 Combining equations (5) and (4) yields for the net mass outflow through the control volume  
 142 in Figure 1 as (to the order of  $(\Delta x_i)^{\beta_{x_i}}$ ,  $i=1,2$ ),

$$\begin{aligned} & \frac{\Gamma(2-\beta_{x_1})}{x_1^{1-\beta_{x_1}}} \left(\frac{\partial}{\partial x_1}\right)^{\beta_{x_1}} \left(\rho Q_{x_1}(\bar{x}; t)\right) \Delta x_1 \Delta x_2 + \\ & \frac{\Gamma(2-\beta_{x_2})}{x_2^{1-\beta_{x_2}}} \left(\frac{\partial}{\partial x_2}\right)^{\beta_{x_2}} \left(\rho Q_{x_2}(\bar{x}; t)\right) \Delta x_1 \Delta x_2 - \rho q_v \Delta x_1 \Delta x_2, \quad \bar{x} = (x_1, x_2). \end{aligned} \quad (6)$$

143 Denoting the water volume within the control volume in Figure 1 by  $V_w$  and using the concept  
 144 of specific yield (effective porosity)  $S_y$  of a phreatic aquifer (Bear and Verruijt, 1987)

$$145 \quad S_y = \frac{\Delta V_w}{\Delta h} \frac{1}{\Delta x_1 \Delta x_2} \quad , \quad (7)$$

146 where  $\Delta V_w$  is the change in water volume in the control volume per change  $\Delta h$  in the hydraulic  
 147 head (the elevation of the phreatic surface (water table) above the flat bottom of the aquifer), the  
 148 time rate of change of mass within the control volume in Figure 1 may be written as (Bear and  
 149 Verruijt, 1987)

$$\frac{S_y(\rho h(\bar{x}; t) - \rho h(\bar{x}; t - \Delta t))}{\Delta t} \Delta x_1 \Delta x_2 \quad (8)$$

150 which can then be expressed in terms of the approximation (2) with respect to the time dimension  
 151 as,

$$152 \quad \frac{S_y}{\Delta t} \left[ \frac{\Delta t^\alpha}{\Gamma(\alpha+1)} \left(\frac{\partial}{\partial t}\right)^\alpha (\rho h) \right] \Delta x_1 \Delta x_2 \quad . \quad (9)$$

153  
 154 To  $\alpha$ -order fractional increments in time (Kavvas et al. 2017b)

$$(\Delta t)^\alpha = \frac{\Gamma(\alpha+1)\Gamma(2-\alpha)}{t^{1-\alpha}} \Delta t \quad . \quad (10)$$

156 Substituting equation (10) into equation (9), one can obtain the time rate of change of mass in the  
 157 control volume, as shown in Figure 1;

158



$$S_y \frac{\Gamma(2-\alpha)}{t^{1-\alpha}} \left(\frac{\partial}{\partial t}\right)^\alpha (\rho h) \Delta x_1 \Delta x_2. \quad (11)$$

159

160 As the time rate of change of mass within the control volume, as shown in Figure 1, must be  
 161 inversely proportional to the net mass flux passing through the control volume, one may combine  
 162 Equations (6) and (11) to obtain

163

$$164 \left[ \frac{\Gamma(2-\beta_{x_1})}{x_1^{1-\beta_{x_1}}} \left(\frac{\partial}{\partial x_1}\right)^{\beta_{x_1}} (\rho Q_{x_1}(\bar{x}; t)) + \frac{\Gamma(2-\beta_{x_2})}{x_2^{1-\beta_{x_2}}} \left(\frac{\partial}{\partial x_2}\right)^{\beta_{x_2}} (\rho Q_{x_2}(\bar{x}; t)) - \rho q_v \right] \Delta x_1 \Delta x_2 =$$

$$165 - S_y \frac{\Gamma(2-\alpha)}{t^{1-\alpha}} \left(\frac{\partial}{\partial t}\right)^\alpha (\rho h) \Delta x_1 \Delta x_2 \quad (12)$$

166

$$167 \frac{\Gamma(2-\beta_{x_1})}{x_1^{1-\beta_{x_1}}} \left(\frac{\partial}{\partial x_1}\right)^{\beta_{x_1}} (\rho Q_{x_1}(\bar{x}; t)) + \frac{\Gamma(2-\beta_{x_2})}{x_2^{1-\beta_{x_2}}} \left(\frac{\partial}{\partial x_2}\right)^{\beta_{x_2}} (\rho Q_{x_2}(\bar{x}; t)) - \rho q_v = - S_y \frac{\Gamma(2-\alpha)}{t^{1-\alpha}} \left(\frac{\partial}{\partial t}\right)^\alpha (\rho h) \quad (13)$$

168 for  $0 < \alpha, \beta_{x_1}, \beta_{x_2} < 1$ ,  $\bar{x} = (x_1, x_2)$ .

169 Within the framework of fluid incompressibility in the unconfined aquifer, equation (13)  
 170 reduces further to

171

$$172 \frac{\Gamma(2-\beta_{x_1})}{x_1^{1-\beta_{x_1}}} \left(\frac{\partial}{\partial x_1}\right)^{\beta_{x_1}} (Q_{x_1}(\bar{x}; t)) + \frac{\Gamma(2-\beta_{x_2})}{x_2^{1-\beta_{x_2}}} \left(\frac{\partial}{\partial x_2}\right)^{\beta_{x_2}} (Q_{x_2}(\bar{x}; t)) - q_v = - S_y \frac{\Gamma(2-\alpha)}{t^{1-\alpha}} \frac{\partial^\alpha h}{(\partial t)^\alpha}$$

173

$$174 \frac{\Gamma(2-\beta_{x_1})}{\Gamma(2-\alpha)} \frac{t^{1-\alpha}}{x_1^{1-\beta_{x_1}}} \left(\frac{\partial}{\partial x_1}\right)^{\beta_{x_1}} (Q_{x_1}(\bar{x}; t)) + \frac{\Gamma(2-\beta_{x_2})}{\Gamma(2-\alpha)} \frac{t^{1-\alpha}}{x_2^{1-\beta_{x_2}}} \left(\frac{\partial}{\partial x_2}\right)^{\beta_{x_2}} (Q_{x_2}(\bar{x}; t)) - \frac{t^{1-\alpha}}{\Gamma(2-\alpha)} q_v$$

$$175 = - S_y \frac{\partial^\alpha h}{(\partial t)^\alpha} \quad (14)$$

176

177 for  $0 < \alpha, \beta_{x_1}, \beta_{x_2} < 1$ ,  $\bar{x} = (x_1, x_2)$  as the time-space fractional continuity equation of transient  
 178 groundwater flow in an anisotropic unconfined aquifer with multi-fractional dimensions and in  
 179 fractional time.

180 Performing a dimensional analysis of Equation (14) yields

$$\frac{L}{T^\alpha} = \frac{T^{1-\alpha}}{L^{1-\beta_{x_1}}} \frac{1}{L^{\beta_{x_1}}} \frac{L^2}{T} = \frac{T^{1-\alpha}}{L^{1-\beta_{x_2}}} \frac{1}{L^{\beta_{x_2}}} \frac{L^2}{T} = \frac{T^{1-\alpha}}{1} \frac{L}{T} = \frac{1}{L^{1-\beta_z}} \frac{1}{L^{\beta_z}} \frac{L}{T} = \frac{L}{T^\alpha} \quad (15)$$



181 where L denotes length and T denotes time. Hence, the left-hand and right-hand sides of the  
 182 continuity equation (14) for transient groundwater flow in an unconfined aquifer in multi-  
 183 fractional space and fractional time are consistent as shown in equation (15).

184 For  $n-1 < \alpha, \beta_{x_i} < n$  where n is any positive integer, as  $\alpha$  and  $\beta_{x_i} \rightarrow n$ , the Caputo fractional  
 185 derivative of a function  $f(y)$  to order  $\alpha$  or  $\beta_{x_i}$  ( $i = 1, 2$ ) yields the standard n-th derivative of the  
 186 function  $f(y)$  (Podlubny, 1998). When  $\alpha$  and  $\beta_{x_i} \rightarrow 1$  ( $i = 1, 2$ ), the continuity equation (14)  
 187 becomes the conventional continuity equation for transient groundwater flow in an unconfined  
 188 aquifer:

$$-S_y \frac{\partial h}{\partial t} = \frac{\partial}{\partial x_1} (Q_{x_1}(\bar{x}; t)) + \frac{\partial}{\partial x_2} (Q_{x_2}(\bar{x}; t)) - q_v. \quad (16)$$

### 189 3. Motion Equation (Specific Discharge) in Fractional Multi-Dimensional Unconfined 190 Aquifers

191 Recently, Kavvas et al., (2017a, 2017b) derived a governing equation for water flux  
 192 (specific discharge),  $q_{x_i}$ , ( $i = 1, 2, 3$ ) in a saturated or unsaturated porous medium with fractional  
 193 dimensions in the form,

$$q_i(\bar{x}, t) = -K_{s,x_i}(\bar{x}) \frac{\Gamma(2-\beta_{x_i})}{x_i^{1-\beta_{x_i}}} \frac{\partial^{\beta_{x_i}} h}{(\partial x_i)^{\beta_{x_i}}}, \quad i = 1, 2, 3; \quad \bar{x} = (x_1, x_2, x_3). \quad (17)$$

194 where  $K_{s,x_i}(\bar{x})$  is the saturated hydraulic conductivity in the i-th spatial direction ( $i=1,2,3$ ).  
 195 Meanwhile, under the Dupuit approximation of essentially horizontal unconfined aquifer flow  
 196 (water table slope very small) (Bear, 1979), referring to Figure 1, the discharge per unit width in  
 197 the i-th direction ( $i = 1, 2$ ) can be expressed as

$$198$$

$$199 \quad Q_{x_i}(\bar{x}, t) = h q_i(\bar{x}, t), \quad i = 1, 2 \quad ; \quad \bar{x} = (x_1, x_2, ). \quad (18)$$

200

201 Then combining equations (18) and (17) results in

202

$$203 \quad Q_{x_i}(\bar{x}, t) = -K_{s,x_i}(\bar{x}) \frac{\Gamma(2-\beta_{x_i})}{x_i^{1-\beta_{x_i}}} h \frac{\partial^{\beta_{x_i}} h}{(\partial x_i)^{\beta_{x_i}}}, \quad i = 1, 2; \quad \bar{x} = (x_1, x_2, ) \quad (19)$$

204





205 as the governing equation of groundwater motion within an unconfined aquifer with a flat bottom  
 206 confining layer. In equation (19)  $h$  is the unconfined aquifer thickness or the phreatic surface  
 207 elevation above the bottom confining layer.

208 A dimensional analysis on equation (19) yields  $L^2/T$  for the units of both the left-hand-side  
 209 (LHS) and the RHS of the equation, establishing its dimensional consistency.

210 Applying the above-mentioned result of Podlubny (1998) on the convergence of a fractional  
 211 derivative to a corresponding integer derivative for  $\beta_{x_i} \rightarrow 1$  ( $i = 1, 2$ ) reduces the fractional motion  
 212 equation (19) for unconfined groundwater flow to the conventional equation (Bear, 1979):

$$Q_{x_i}(\bar{x}, t) = -K_{s,x_i}(\bar{x})h \frac{\partial h(\bar{x}, t)}{\partial x_i}, \quad i=1,2 \quad (20)$$

213 for the case of integer spatial dimensions. As such, the fractional motion equation (19) for  
 214 unconfined groundwater flow in fractional spatial dimensions is consistent with the conventional  
 215 motion equation for the integer spatial dimensions.

#### 216 4. The Complete Equation for Transient Unconfined Groundwater Flow in Multi-Fractional 217 Space and Fractional Time

218 Combining the fractional motion equation (19) of groundwater flow in an unconfined aquifer  
 219 with the fractional continuity equation (14) of unconfined groundwater flow results in the equation,

220

$$S_y \frac{\partial^\alpha h}{(\partial t)^\alpha} = \frac{\Gamma(2-\beta_{x_1})}{x_1^{1-\beta_{x_1}}} \left( \frac{\partial}{\partial x_1} \right)^{\beta_{x_1}} \left( K_{s,x_1}(\bar{x}) \frac{t^{1-\alpha}}{x_1^{1-\beta_{x_1}}} \frac{\Gamma(2-\beta_{x_1})}{\Gamma(2-\alpha)} h \frac{\partial^{\beta_{x_1}} h}{(\partial x_1)^{\beta_{x_1}}} \right) +$$

$$\frac{\Gamma(2-\beta_{x_2})}{x_2^{1-\beta_{x_2}}} \left( \frac{\partial}{\partial x_2} \right)^{\beta_{x_2}} \left( K_{s,x_2}(\bar{x}) \frac{t^{1-\alpha}}{x_2^{1-\beta_{x_2}}} \frac{\Gamma(2-\beta_{x_2})}{\Gamma(2-\alpha)} h \frac{\partial^{\beta_{x_2}} h}{(\partial x_2)^{\beta_{x_2}}} \right) + \frac{t^{1-\alpha}}{\Gamma(2-\alpha)} q_v \quad (21)$$

223

224 for  $0 < \alpha, \beta_{x_1}, \beta_{x_2} < 1$ ,  $\bar{x} = (x_1, x_2)$  as the time-space fractional governing equation of transient  
 225 unconfined groundwater flow in an anisotropic medium.

226 Performing a dimensional analysis of Equation (21) yields

$$\frac{L}{T^\alpha} = \frac{1}{L^{1-\beta_{x_1}}} \frac{1}{L^{\beta_{x_1}}} \frac{L}{T} \frac{T^{1-\alpha}}{L^{1-\beta_{x_1}}} L \frac{L}{L^{\beta_{x_1}}} = \frac{1}{L^{1-\beta_{x_2}}} \frac{1}{L^{\beta_{x_2}}} \frac{L}{T} \frac{T^{1-\alpha}}{L^{1-\beta_{x_2}}} L^2 \frac{L^2}{L^{\beta_{x_2}}} = \frac{T^{1-\alpha}}{1} \frac{L}{T} = \frac{L}{T^\alpha} \quad (22)$$



227 where  $L$  denotes length and  $T$  denotes time. Hence, the left-hand and right-hand sides of the  
228 governing equation (21) for transient groundwater flow in an unconfined aquifer in multi-  
229 fractional space and fractional time are consistent.

230 Specializing the above-discussed result of Podlubny (1998) to  $n = 1$ , for  $\alpha$  and  $\beta_{x_i} \rightarrow 1$  ( $i =$   
231  $1, 2$ ) reduces the governing fractional equation (21) to the conventional governing equation for  
232 transient groundwater flow in an unconfined aquifer (Bear, 1979):

$$233 \quad S_y \frac{\partial h}{\partial t} = \frac{\partial}{\partial x_1} \left( K_{s,x_1}(\bar{x}) h \frac{\partial h(\bar{x},t)}{\partial x_1} \right) + \frac{\partial}{\partial x_2} \left( K_{s,x_2}(\bar{x}) h \frac{\partial h(\bar{x},t)}{\partial x_2} \right) + q_v \quad (23)$$

234

## 235 5. Numerical application

236

237 To demonstrate the skills of the proposed fractional governing equation of unconfined aquifer  
238 groundwater flow, a numerical application is performed using the proposed fractional governing  
239 equation to the physical setting of an example from Wang and Anderson (1995), as depicted in  
240 Figure 2. The numerical problem of seepage through a dam under a sudden change in the water  
241 surface elevation at the downstream section of the dam is modified based on seepage through a  
242 dam, Page 53 and Problem 4.4 (a), Page 89 in Wang and Anderson (1995), as shown in Figure 2.  
243 The water seepage through the dam's body may be interpreted as one-dimensional groundwater  
244 flow through an unconfined aquifer. The unconfined flow system locates the top boundary of the  
245 saturated zone in an earthen dam and the bottom of the dam rests on impermeable rock. For this  
246 example, the unconfined aquifer length  $L$  is 100 m. The initial water level in the upstream and  
247 downstream sections of the dam and through the dam's body is 16 m. Then immediately after the  
248 initial time, the water level at the downstream section of the dam is suddenly dropped to 11 m and  
249 remains as 11 m afterwards. The unconfined aquifer parameters, storage coefficient and hydraulic  
250 conductivity, are  $S = 0.2$ ,  $K=0.002$  m/min respectively. The analytical solution for this problem at  
251 the steady-state is:

$$252 \quad h = \sqrt{\frac{h_2^2 - h_1^2}{L} x} + h_1^2 \quad (24)$$

253 where  $h$  is the depth of the unconfined groundwater surface from the bottom layer;  $L$  is the aquifer  
254 length;  $x$  is the distance from the upstream location of the dam body, and  $h_1$  and  $h_2$  are as shown  
255 in Figure 2.



256 In Figure 3(a) the normalized groundwater head  $h/h_1$  at location  $x=L/2$  through time under  
257 different fractional power values is shown. Meanwhile, Figure 3(b) shows the normalized  
258 groundwater head  $h/h_1$  at the time instance  $t=15000$  min as function of location throughout the  
259 dam's body, and the analytical solution of the standard governing equation of unconfined  
260 groundwater flow when  $\beta_x = \alpha = 1$  at the steady state. The considered fractional derivative  
261 powers in space and time are  $\beta_x = \alpha = 0.7, 0.8, 0.9, 1.0$ . As can be seen from Figure 3(a), the  
262 hydraulic head recession in time slows down with the decrease of  $\beta_x = \alpha$  from 1. The hydraulic  
263 heads in Figure 3(a) have heavier tails as orders of time and space fractional derivatives decrease  
264 from 1 towards 0.7. Meanwhile, Figure 3(b) shows that the numerical solution of the governing  
265 fractional equation at  $\beta_x = \alpha = 1.0$  and at a very long time after the initial condition, matches  
266 perfectly the steady state analytical solution (24) of the standard equation (23) with the specified  
267 initial/boundary conditions.

## 268 6. Discussion

269 From the standard governing equation (23) of unconfined groundwater flow in integer time-  
270 space the saturated hydraulic conductivity may be interpreted as a diffusion coefficient for the  
271 nonlinear diffusion of groundwater in an unconfined aquifer. The basic difference between  
272 confined and unconfined groundwater flow is that the former can be interpreted as a linear  
273 diffusion of groundwater while the latter is a nonlinear diffusion of groundwater within an aquifer.  
274 Similar to saturated hydraulic conductivities in equation (26) in Kavvas et al., (2017a) for the  
275 fractional confined aquifer groundwater flow, the saturated hydraulic conductivities in equation  
276 (21) above, which governs fractional unconfined aquifer groundwater flow, are modulated by the  
277 ratios of fractional time to fractional space,  $\frac{t^{1-\alpha}}{x_i^{1-\beta x_i}}$ ,  $i=1,2$ . In other words, the confined and  
278 unconfined groundwater diffusion in fractional time-space is modulated by the above fractional  
279 time-space ratios.

280 Numerical application demonstrated that as the powers of the space and time fractional  
281 derivatives decrease from 1, the recession rate of the nondimensional groundwater hydraulic heads  
282 slows down when compared to the case by the conventional governing equation (i.e., with integer  
283 order derivatives). This behavior also indicates the modulation of the nonlinear diffusion of the  
284 groundwater by the fractional powers of time and space.



285 As mentioned in the Introduction section, unconfined groundwater flow is the fundamental  
286 component of the watershed runoff baseflow since it is the fundamental contributor to the network  
287 streamflow within a watershed during dry periods. As such, the behavior of unconfined  
288 groundwater flow is key to the physically-based understanding of the long memory in watershed  
289 runoff. As seen from the numerical example in Figure 3, the powers of the fractional derivatives  
290 in time and space can modulate the speed of the groundwater table evolution. Hence, they can  
291 modulate the memory of the unconfined aquifer flow, which, in turn, can modulate the memory of  
292 the watershed baseflow. Meanwhile, the Caputo derivative, as defined in its special form  
293  $D_0^{\beta x_i} f(x_i)$  in space in this study, was shown by Kavvas and Ercan (2017) to be a nonlocal quantity  
294 where the effect of the boundary conditions on the groundwater flow within the flow domain can  
295 have long spatial memories with the decrease in the powers of the spatial fractional derivatives  
296 from unity. Similarly, it was shown by Kavvas et al. (2017a) that the Caputo derivative in time,  
297  $D_0^\alpha f(t)$ , as defined in this study, is nonlocal in time, and can carry the effect of initial conditions  
298 on the groundwater flow for long times as the power in the time fractional derivative decreases  
299 from 1. Therefore, the fractional governing equation of unconfined groundwater flow in fractional  
300 time and multi-fractional space has the potential to describe the long memory characteristics of  
301 baseflow within a watershed. This important topic shall be explored in the near future.

302

## 303 7. Conclusion

304

305 A dimensionally-consistent fractional governing equation of transient unconfined aquifer  
306 groundwater flow was derived within fractional differentiation framework. After developing a  
307 fractional continuity equation, a previously-developed dimensionally consistent equation for water  
308 flux in unsaturated/saturated porous media was combined with the Dupuit approximation to obtain  
309 an equation for groundwater motion in multi-fractional space in unconfined aquifers. Combining  
310 the fractional continuity and motion equations, the governing equation of transient unconfined  
311 aquifer groundwater flow in a multi-fractional medium in fractional time was then obtained. To  
312 demonstrate the skills of the proposed fractional governing equation of unconfined aquifer  
313 groundwater flow, a numerical application was presented. As demonstrated in the numerical  
314 application results, the orders of the fractional space and time derivatives modulate the speed of  
315 groundwater table evolution, slowing the process with decrease in the powers of the fractional



316 derivatives from 1. It is also shown that the proposed dimensionally consistent fractional governing  
317 equations approach to the corresponding conventional equations as the fractional orders of the  
318 derivatives go to 1.

319

#### 320 **Data availability.**

321 The data used in this article can be accessed by contacting the corresponding author.

322

#### 323 **Competing interests.**

324 The authors declare that they have no conflict of interest.

325

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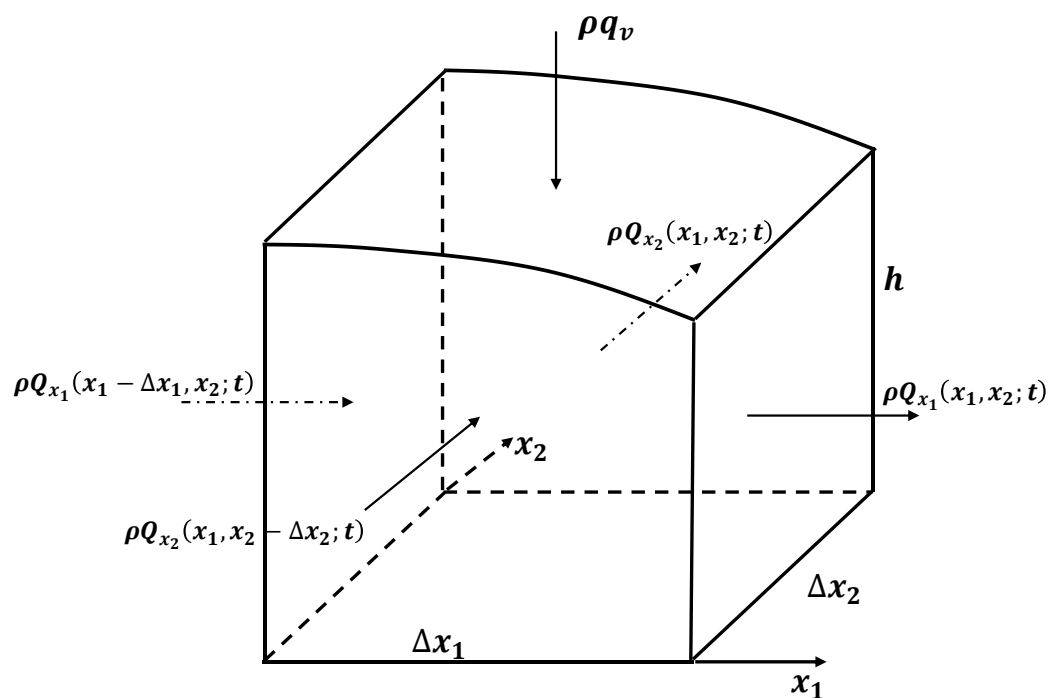


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416 Figure 1. The mass flux through the control volume of an unconfined aquifer.

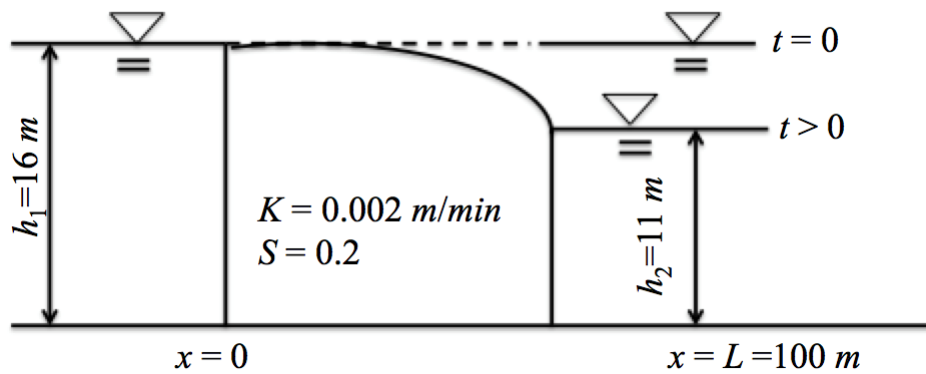
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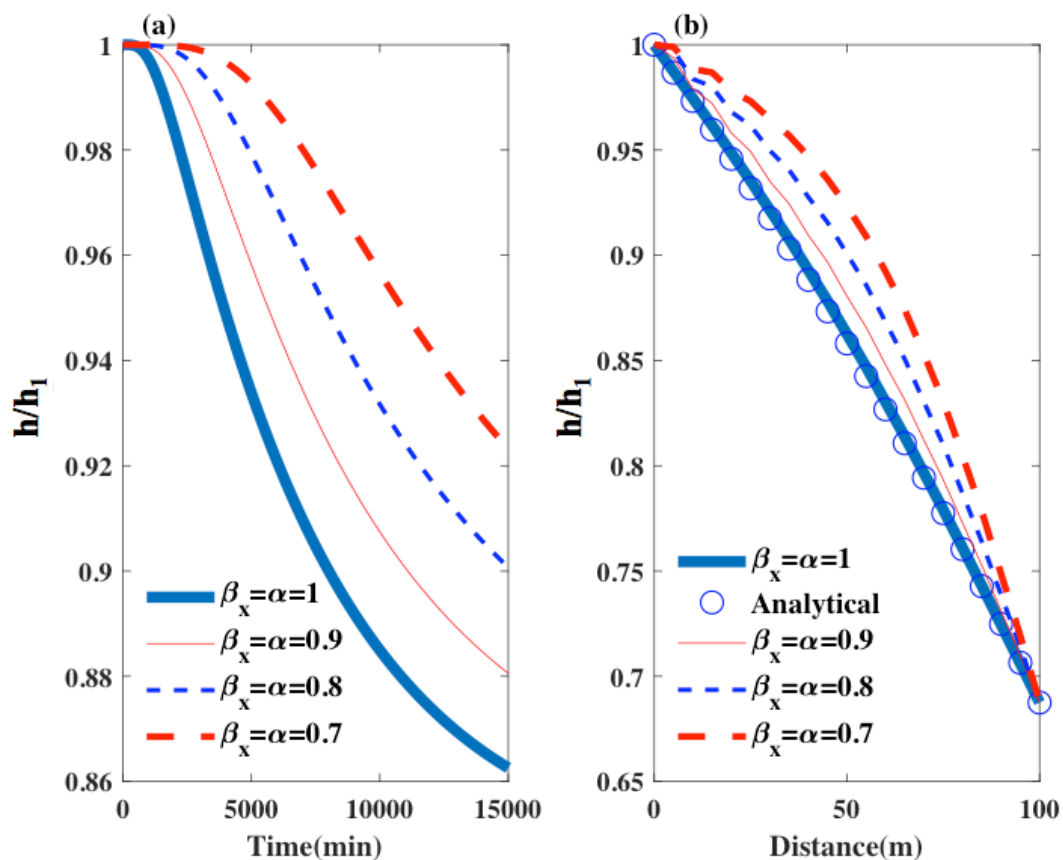


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Figure 2. The sketch of the problem of the water seepage through a dam's body as an unconfined groundwater flow



427

428 Figure 3. (a) The normalized groundwater head  $h/h_1$  at  $x=L/2$  through time under different  
 429 fractional derivative powers; (b) The normalized groundwater head  $h/h_1$  at  $t=15000$  min through  
 430 length of the aquifer (through the body of the dam) and the analytical solution of the standard  
 431 governing equation of unconfined groundwater flow when  $\beta_x = \alpha = 1$  at the steady state.  
 432